#### Singapore Standard SS EN 1998-1: 2013

# Eurocode 8: Design of structures for earthquake resistance – Part 1: General rules, seismic actions and rules for buildings

#### **AMENDMENT NO. 1**

May 2021

#### 1. Page 71, 4.4.1, General

Add the following new Paragraph (3):

(3) For low-dissipative structures (see **2.2.2(2)**), the ductility, capacity design and overstrength requirements of 4.4.2 do not need to be applied.

#### 2. Page 89, 5.4.1.2.2, Columns

Replace Paragraph (1) with the following:

- (1) The minimum cross-sectional dimension of primary seismic columns shall be not less than 200 mm.
- (2) Unless  $\theta \le 0.1$  (see **4.4.2.2(2)**), the cross-sectional dimensions of primary seismic columns should not be smaller than:
- one twentieth of the larger distance between the point of contraflexure of the deflected shape and the ends of the column, for bending within a plane parallel to the column dimension considered;
- 250 mm.

### 3. Page 131, 5.11.2.1.2, Overdesigned connections

Replace Paragraph (1) with the following:

(1) The design action-effects of overdesigned connections should be derived on the basis of the capacity design rules of **5.4.2.2** for beams and **5.4.2.3** for columns, on the basis of overstrength flexural resistances at the end sections of critical regions equal to  $\gamma_{Rd}$ . M<sub>Rd</sub>, with the factor  $\gamma_{Rd}$  taken as being equal to 1,20 for DCM and to 1,35 for DCH.

# 4. Page 132, 5.11.3.2, Columns

Replace Paragraph (3) with the following:

(3) For precast frame systems with hinged column-to-beam connections, the columns should be fully fixed against translation and rotation at the base in foundations designed in accordance with **5.11.2.1.2**.

## 5. Page 135, 5.11.3.5, Diaphragms

Replace Paragraph (6) with the following:

(6) In-plane acting shear forces along slab-to-slab or slab-to-beam connections should be computed in accordance with **4.4.2.5**. The design resistance should be computed as in **5.11.2.2**.